

Some aspects of structural engineering of dagger boards.

Martin Schön, Dec. 2004

Foreword

This is not ment to be an exhaustive treatment of the structural engineering of dagger boards. My intention is only to give you some insight into how complex even the structural engineering of dagger boards is. I have only dealt with two load cases and I don't take the shear stiffness of the web into consideration. A carbon/epoxy design is studied since these materials are not prohibitively expensive any more.

Disclaimer

Material data used here should be regarded as examples.

First load case: Hard on the wind, dagger boards fully down

$$RM := 30 \cdot 10^3 \cdot \text{newton} \cdot \text{m}$$

Righting moment from stability calc.

$$CSA := 5.5 \cdot \text{m}$$

Height of centre of effort of sail plan.

$$FRM := \frac{RM}{CSA}$$

Side force countered by board. I think it is reasonable to have one board take this load alone. Wave action is the reason.

Geometry and Load Input

Length in water: $l := 1.4 \cdot \text{m}$

Length in box: $d := 1.0 \cdot \text{m}$

Thickness: $H := 60 \cdot \text{mm}$

Cord: $c := 0.45 \cdot \text{m}$

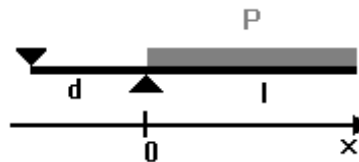
Area: $A := l \cdot c$

$$A = 0.63 \cdot \text{m}^2$$

Load from stab.: $PRM := \frac{FRM}{A}$

$$PRM = 8.658 \cdot 10^3 \cdot \text{Pa}$$

Water density: $\rho := 1 \cdot \frac{\text{kg}}{\text{liter}}$



Material data, unidirectional carbon in epoxy

Young's modulus: $E := 62 \cdot 10^9 \cdot \text{Pa}$

Ultimate stress: $R_p := 620 \cdot 10^6 \cdot \text{Pa}$

Strain at fatigue stress: $s_u := 0.3 \cdot \%$

Fatigue stress: $R_u := E \cdot s_u$

$$R_u = 1.86 \cdot 10^8 \cdot \text{Pa}$$

This data is taken from "Marine Composites" by Eric Greene Associates and I use data for compression loads since this is the worst data.

The material builds 0.53 mm for each 350 g layer.

The ultimate strain for carbon fibres is some 1% and for

good epoxy I find values between 3% and 6%. I have used 10x safety margin on 3% to calculate an acceptable stress level (fatigue stress). This is not very scientific!!!

Here I just create a convenient variable x that I sweep along the board length.

$$J := 241 \quad j := 0 .. J \quad d + l = 2.4 \cdot \text{m} \quad x_j := -d + \frac{d + l}{J} \cdot j$$

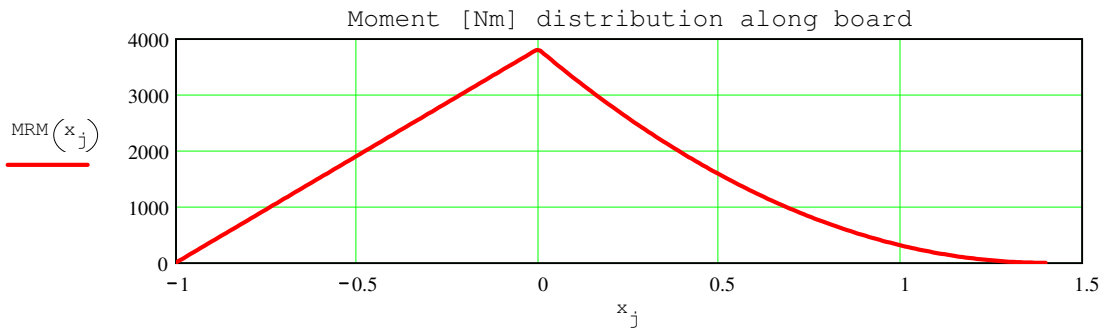
Structural 'textbook' equations

$$\text{Moment: } MRM(x) := \begin{cases} \frac{c \cdot PRM \cdot (l - x)^2}{2} & \text{if } x > 0 \\ \frac{c \cdot PRM \cdot l^2}{2} \cdot \frac{d + x}{d} & \text{otherwise} \end{cases}$$

Outside box

$$MRM(0 \cdot \text{m}) = 3.818 \cdot 10^3 \cdot \text{m} \cdot \text{newton}$$

Inside box



Loads on the dagger board case

Total side-load from righting moment: $FRM = 5.455 \cdot 10^3 \text{ } \circ_{\text{newton}}$

Load at upper end of board: $F_u := \frac{MRM(0 \cdot \text{m})}{d} \quad F_u = 3.818 \cdot 10^3 \text{ } \circ_{\text{newton}}$

Load at hull bottom: $F_b := FRM + F_u \quad F_b = 9.273 \cdot 10^3 \text{ } \circ_{\text{newton}}$

Note The load at the lower end of the dagger board case is equivalent to having the hull laying down on its side and putting a 944 kg weight at that case exit. Make sure the boat is stronger than the boards!

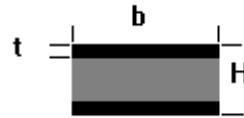
Engineering of board structural member

Beam width: $b := 10 \cdot \text{cm}$ Laminate thickness: $t := 4.24 \cdot \text{mm}$ (guess)

Sectional stiffness: $I := \frac{b \cdot [H^3 - (H - 2 \cdot t)^3]}{12}$

Sectional strength: $w := \frac{I \cdot 2}{H}$

Max stress in design above: $\sigma_{\text{max}} := \frac{MRM(0 \cdot \text{m})}{w} \quad \sigma_{\text{max}} = 1.734 \cdot 10^8 \text{ } \circ_{\text{Pa}}$

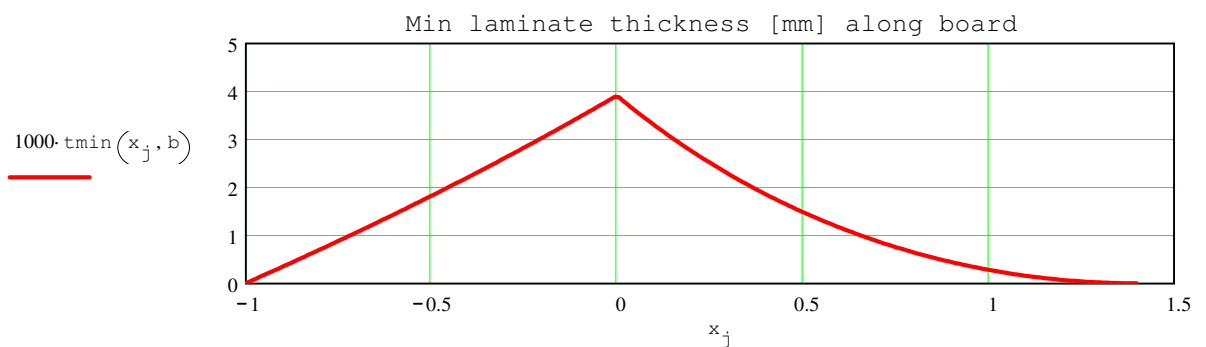


Calculating required minimum laminate thickness

by going from moment and strength via section properties:

From RM: $w_{\text{min}}(x) := \frac{MRM(x)}{R_u} \quad I_{\text{min}}(x) := H \cdot \frac{w_{\text{min}}(x)}{2}$

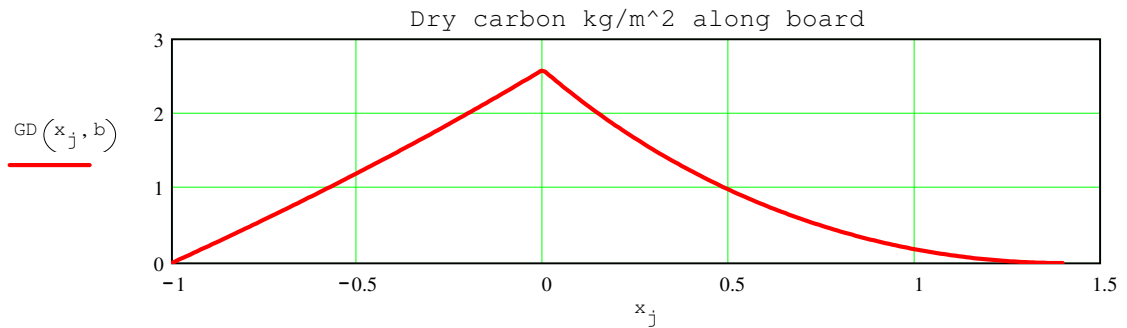
$$t_{\text{min}}(x, b) := 0.5 \cdot \left[H - \left(H^3 - \frac{12 \cdot I_{\text{min}}(x)}{b} \right)^{\frac{1}{3}} \right] \quad t_{\text{min}}(0 \cdot \text{m}, b) = 3.908 \cdot 10^{-3} \cdot \text{m}$$



Fibreweight/m² calculation

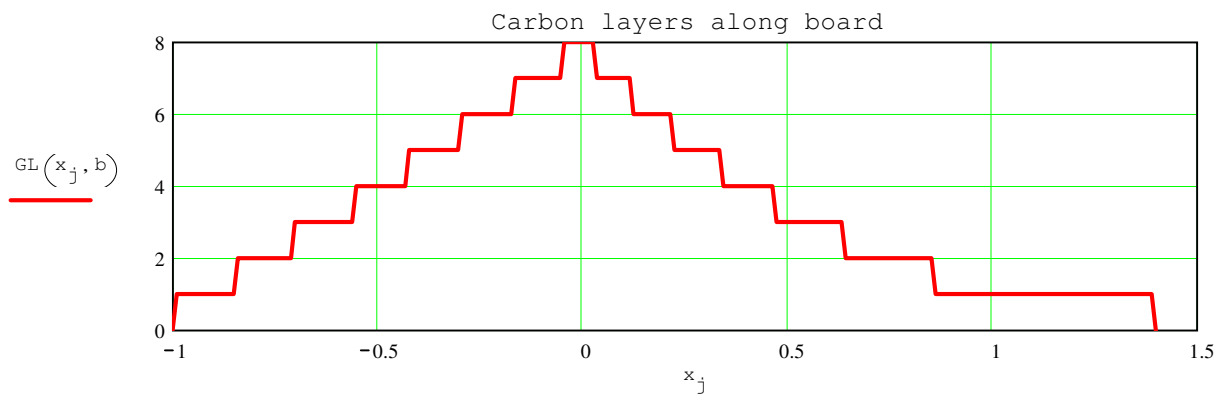
Assumed thickness ' density' :

$$TGD := \frac{1.51 \cdot 10^{-3} \cdot m}{1 \cdot \frac{kg}{m^2}} \quad GD(x, b) := \frac{tmin(x, b)}{TGD}$$



Calculating how many layers that are required

$$fibreweight := 0.35 \cdot kg \cdot m^{-2} \quad GL(x, b) := \text{ceil} \left[\frac{\text{Re}((GD(x, b)))}{fibreweight} \right]$$



Shear web compression at hull exit

Whatever keeps the two sides of the board apart must be hard enough not to crush at the hull exit.

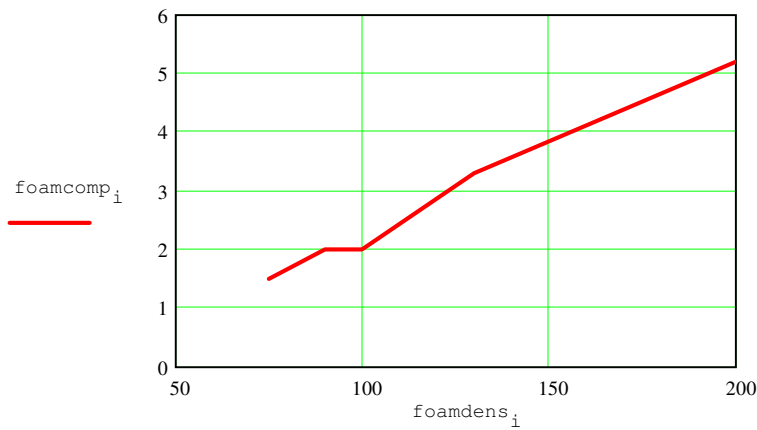
Pressure assuming some 5 cm of the beam length is really involved:

$$P_{web}(b) := \frac{Fb}{b \cdot 50 \cdot mm} \quad P_{web}(b) = 1.855 \cdot 10^6 \text{ Pa}$$

$$i := 0..4$$

$$foamdens := (75 \ 90 \ 100 \ 130 \ 200)^T \text{ kg/m}^3 \quad \text{Termanto (PVC foam)}$$

$$foamcomp := (1.5 \ 2 \ 2 \ 3.3 \ 5.2)^T \text{ MPa}$$



Looks like the 100 kg foam is the lightest we can use. Most woods including end grain balsa would be more than strong enough but must be carefully protected from water. Balsa also has a much higher shear modulus, something that has a considerable impact on the stiffness of the board. NB With 100 kg foam there is not much margin for shock loads etc. I use 130 kg foam in the rest of this document.

Board deflection while going to windward

I now look at the board deflection and take into account the fact that its stiffness is a function of the co-ordinate along the board. I start by calculating the moment of inertia from the laminate as built.

$$GD_{\text{build}}(x, b) := GL(x, b) \cdot \text{fibreweight}$$

$$t_{\text{build}}(x, b) := GD_{\text{build}}(x, b) \cdot \text{TGD}$$

$$I_{\text{build}}(x, b) := \frac{b \cdot [H^3 - (H - 2 \cdot t_{\text{build}}(x, b))^3]}{12}$$

Text book equation for the deflection of a homogenous console beam with uniform load:

$$\frac{\text{FRM} \cdot (1 - x)}{24 \cdot EI} \cdot [4 \cdot l^3 - (1 - x)^3]$$

Now this equation has been differentiated with respect to x and I_build has been introduced:

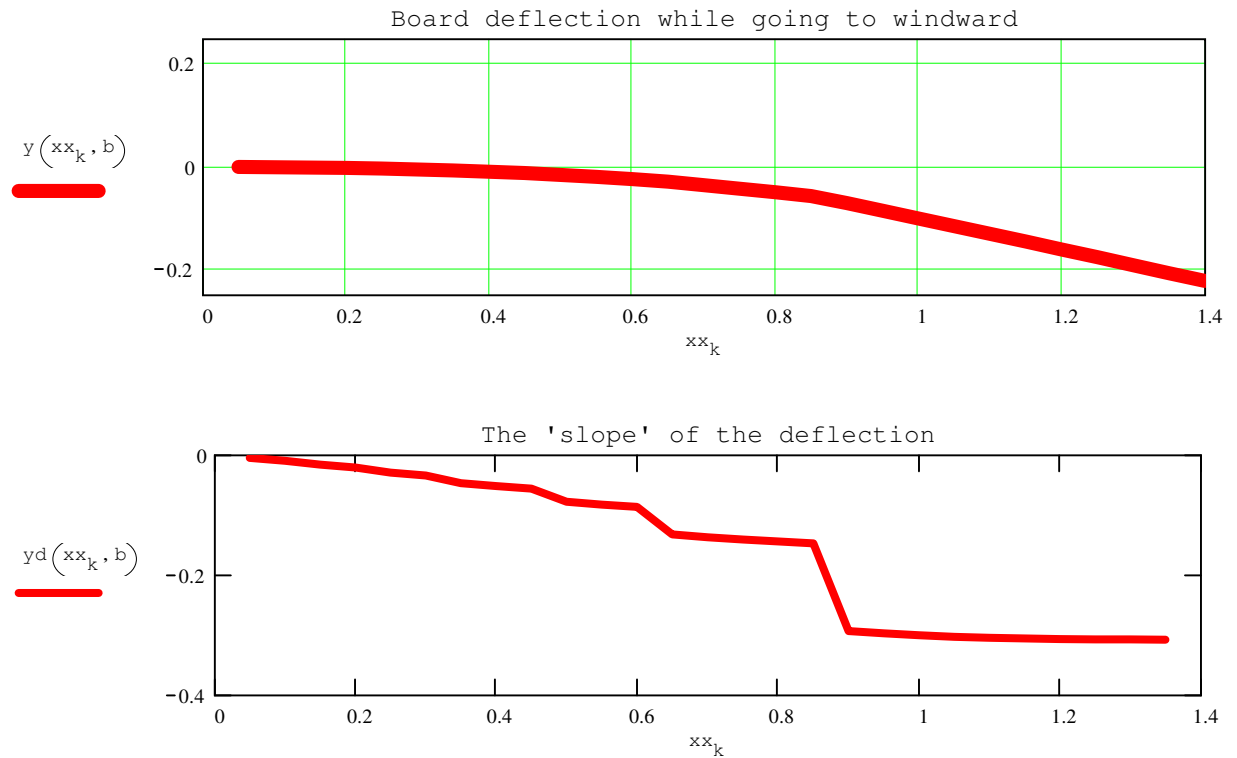
$$y_d(x, b) := \frac{-1}{24} \cdot \frac{\text{FRM}}{E \cdot I_{\text{build}}(x, b) \cdot l} \cdot [4 \cdot l^3 - (1 - x)^3] + \frac{1}{8} \cdot \text{FRM} \cdot \frac{(1 - x)^3}{E \cdot I_{\text{build}}(x, b) \cdot l}$$

Deflection of the board as a function of x now becomes:

$$y(x, b) := \int_0^x \left[\frac{-1}{24} \cdot \frac{\text{FRM}}{E \cdot I_{\text{build}}(x, b) \cdot l} \cdot [4 \cdot l^3 - (1 - x)^3] + \frac{1}{8} \cdot \text{FRM} \cdot \frac{(1 - x)^3}{E \cdot I_{\text{build}}(x, b) \cdot l} \right] dx$$

Board tip deflection: $y(1 - 0.001 \cdot m, b) = -0.222 \cdot m$

$$k := 1 \dots 28 \quad xx_k := 0 + k \cdot 0.05 \cdot m$$



The nonconstant stiffness made this a bit complex and I have yet to take into account the finite shear modulus of the shear web.

Note how uneven the ' slope' curve is. Each step corresponds to a local stress peak. It really pays off to taper the ends of each laminate layer to minimise stress concentration.

The shear stress in the foam while going to windward

Here I calculate the shear stress for the foam in the sandwich. NB This calculation is really only true for a homogenous beam. What happens for tapered beams I must find out.

Shear modulus, shear strength and Young' s modulus of 130 kg foam:

$$G := 41 \cdot 10^6 \cdot \text{Pa} \quad \tau := 2.4 \cdot 10^6 \cdot \text{Pa}$$

$$E_f := 0.19 \cdot 10^9 \cdot \text{Pa}$$

$$\tau_f(b) := \frac{FRM}{H \cdot b} \quad \tau_f(b) = 9.091 \cdot 10^5 \cdot \text{Pa}$$

$$\frac{\tau_f(b)}{\tau} = 37.879 \%$$

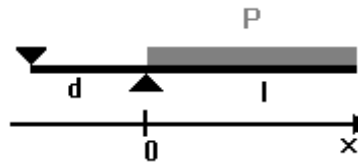
% of ultimate shear strength. Is this enough? I doubt it.

Second load case: broad reaching with boards halfway down

In this scenario we broad reach at 'full speed' with both boards halfway down. Steep waves (wind against tide?) make sure the boards see high and opposing loads.

Length in water: $l := \frac{1}{2}$

Length in box: $d := 1.0 \text{ m}$



Area: $A := l \cdot c$

$A = 0.315 \cdot \text{m}^2$

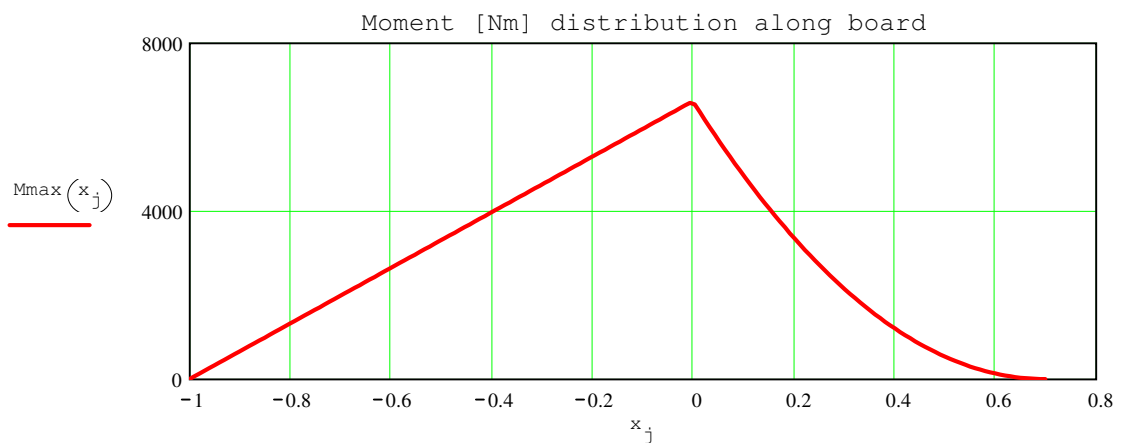
$V_{\max} := 10 \cdot \frac{\text{m}}{\text{sec}}$ $Cl_{\max} := 1.2$ $F_{\max} := 0.5 \cdot A \cdot Cl_{\max} \cdot \rho \cdot V_{\max}^2$ $P_{\max} := \frac{F_{\max}}{A}$

New swept x:

$J := 171$ $j := 0..J$ $d + l = 1.7 \cdot \text{m}$ $x_j := -d + \frac{d + l}{J} \cdot j$

$$M_{\max}(x) := \begin{cases} \frac{c \cdot P_{\max} \cdot (l - x)^2}{2} & \text{if } x > 0 & \text{Outside box} \\ \frac{c \cdot P_{\max} \cdot l^2}{2} \cdot \frac{d + x}{d} & \text{otherwise} & \text{Inside box} \end{cases}$$

$M_{\max}(0 \cdot \text{m}) = 6.615 \cdot 10^3 \cdot \text{m} \cdot \text{newton}$



Total side-load: $F_{\max} = 1.89 \cdot 10^4 \cdot \text{newton}$

Load at upper end of board: $F_{ul} := \frac{M_{\max}(0 \cdot \text{m})}{d}$ $F_{ul} = 6.615 \cdot 10^3 \cdot \text{newton}$

Load at hull bottom: $F_{bl} := F_{\max} + F_{ul}$ $F_{bl} = 2.552 \cdot 10^4 \cdot \text{newton}$

$w_{\min_h}(x) := \frac{M_{\max}(x)}{R_u}$ $I_{\min_h}(x) := H \cdot \frac{w_{\min_h}(x)}{2}$

$t_{\min_h}(x) := 0.5 \cdot \left[H - \left(H^3 - \frac{12 \cdot I_{\min_h}(x)}{b} \right)^{\frac{1}{3}} \right]$ $t_{\min_h}(0 \cdot \text{m}) = 7.763 \cdot 10^{-3} \cdot \text{m}$

It is clear from this that one should not subject the boat to this kind of load and count on it taking it forever unless it is very robust. Hence, at least one of the boards should be raised almost completely if you sail for any length of time at high speed in rough conditions!

Another conclusion one could draw would be to taper the laminate less aggressively towards the bottom end. A side benefit from this is less deflection. For my earlier designs, that were not designed using this kind of 'number crunching', I have used linear tapers for the laminate. This adds almost nothing to the laminate close to the max but it beefs up the lower parts of the board.

Analysis of altered design

A linear laminate taper is used for better 'tip robustness' and less flex. I also widen the structural member to lower the stress on the foam and the dagger board case.

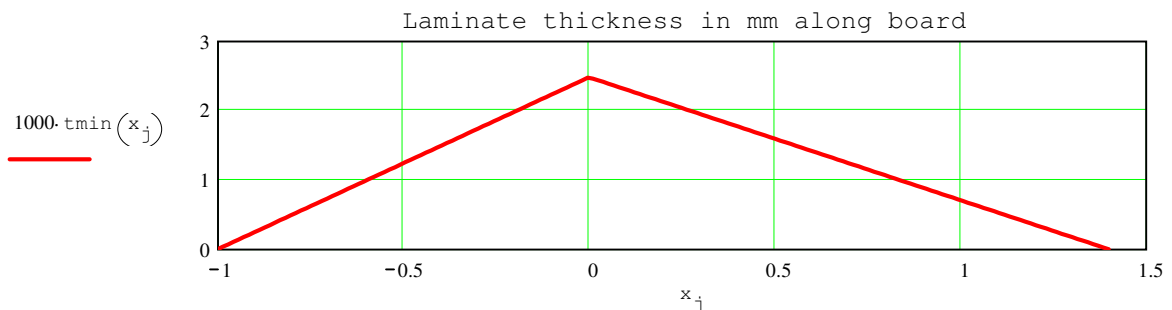
$$R_u := 186 \cdot 10^6 \cdot \text{Pa} \quad b := 0.15 \cdot \text{m} \quad l := 1.4 \cdot \text{m} \quad d := 1.0 \cdot \text{m}$$

$$j := 0..J \quad d + l = 2.4 \cdot \text{m} \quad x_j := -d + \frac{d + l}{J} \cdot j$$

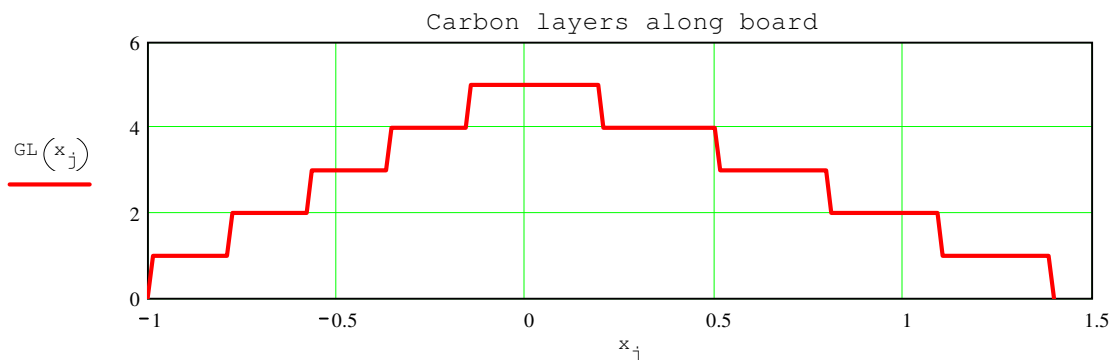
$$J := 241$$

$$w_{\min} := \frac{\text{MRM} (0 \cdot \text{m})}{R_u} \quad I_{\min} := H \cdot \frac{w_{\min}}{2} \quad t_{\min} := 0.5 \cdot \left[H - \left(H^3 - \frac{12 \cdot I_{\min}}{100 \cdot \frac{R_u}{E}} \right)^{\frac{1}{3}} \right] \%$$

$$t_{\min}(x) := \begin{cases} t_{\min} + \frac{t_{\min} \cdot x}{d} & \text{if } x < 0 \\ t_{\min} - \frac{t_{\min} \cdot x}{l} & \text{otherwise} \end{cases}$$



$$GD(x) := \frac{t_{\min}(x)}{\text{TGD}} \quad GL(x) := \text{ceil} \left[\frac{\text{Re}((GD(x)))}{\text{fibweight}} \right]$$



Once again I want to remind you to smoothen the transition where the layers end by not cutting them square. Now, how much will this board flex?

$$GD_{\text{build}}(x) := GL(x) \cdot \text{fibreweight}$$

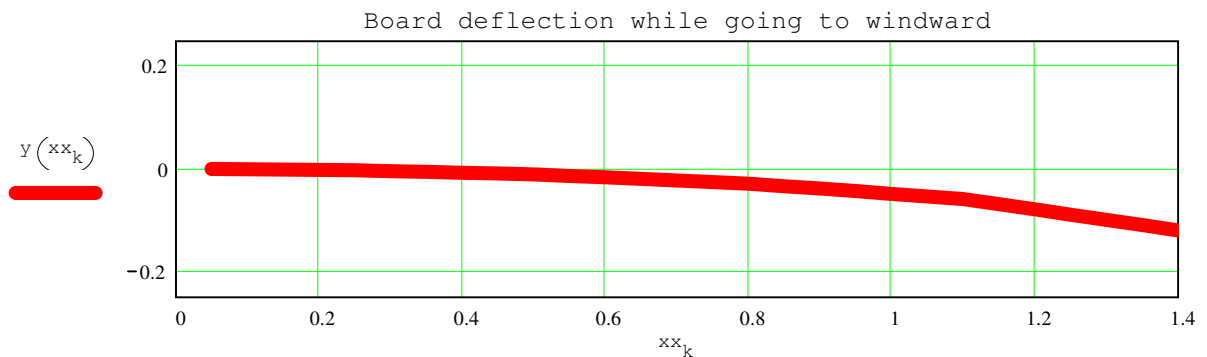
$$t_{\text{build}}(x) := GD_{\text{build}}(x) \cdot TGD$$

$$I_{\text{build}}(x) := \frac{b \cdot [H^3 - (H - 2 \cdot t_{\text{build}}(x))^3]}{12}$$

$$y_d(x) := \frac{-1}{24} \cdot \frac{FRM}{E \cdot I_{\text{build}}(x) \cdot l} \cdot [4 \cdot l^3 - (1-x)^3] + \frac{1}{8} \cdot FRM \cdot \frac{(1-x)^3}{E \cdot I_{\text{build}}(x) \cdot l}$$

$$y(x) := \int_0^x \left[\frac{-1}{24} \cdot \frac{FRM}{E \cdot I_{\text{build}}(x) \cdot l} \cdot [4 \cdot l^3 - (1-x)^3] + \frac{1}{8} \cdot FRM \cdot \frac{(1-x)^3}{E \cdot I_{\text{build}}(x) \cdot l} \right] dx$$

$$y(1 - 0.001 \cdot m) = -0.12 \cdot m \quad \text{Stiffer!}$$



Closing remark

Use cedar, balsa or very tough foam for the shear web. Another alternative is to build a composite I-beam or box-beam. Both Malcolm Tennant and John Shuttleworth use this type of design. Ian Farrier likes balsa. Not only are these materials tougher than a moderately dense PVC foam but they have much higher shear rigidity. I have neglected this aspect of the shear web in my calculations above but it has big impact on the over all stiffness of the boards.